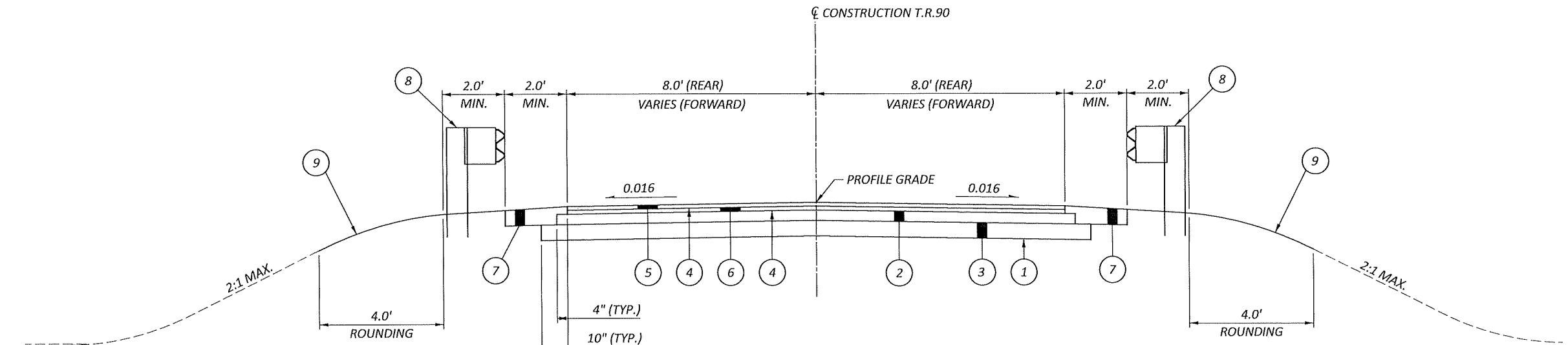


LEGEND

- 1 ITEM 204 - SUBGRADE COMPACTION
- 2 ITEM 301 - 4" ASPHALT CONCRETE BASE, PG64-22
- 3 ITEM 304 - 6" AGGREGATE BASE
- 4 ITEM 407 - TACK COAT (@ 0.05 GAL./SQ.YD.)
- 5 ITEM 441 - 1 $\frac{1}{4}$ " - ASPHALT CONCRETE SURFACE COURSE, TYPE 1, (448) PG64-22
- 6 ITEM 441 - 1 $\frac{1}{4}$ " ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 2, (448)
- 7 ITEM 411 - 6" STABILIZED CRUSHED AGGREGATE
- 8 ITEM 606 - GUARDRAIL, TYPE MGS
- 9 ITEM 659 - SEEDING AND MULCHING

ROADWAY SECTION

REAR
STA. 0+50.00 TO STA. 1+40.00

FORWARD
STA. 2+04.00 TO STA. 2+83.00

ROADWAY TYPICAL SECTIONS



DESIGN AGENCY	
Kingman County Office	
DESIGNER	
SEJ	
REVIEWER	
GJW	3/20/23
PROJECT ID	117331
SHEET	TOTAL
2	18

ROUNDING

THE ROUNDING AT SLOPE BREAKPOINTS SHOWN ON THE TYPICAL SECTIONS APPLIES TO ALL CROSS-SECTIONS, EVEN THOUGH OTHERWISE SHOWN.

UTILITIES

LISTED BELOW ARE ALL UTILITIES LOCATED WITHIN THE PROJECT CONSTRUCTION LIMITS TOGETHER WITH THEIR RESPECTIVE OWNERS:

AEP OHIO POWER:
777 HOPEWELL DRIVE, HEATH, OHIO 43056
ATTN: PAUL PAXTON, 740-348-5322 (PTPAXTON@AEP.COM)

ELEVATION DATUM

ALL ELEVATIONS ARE BASED ON NADV 88 DATUM.

EXISTING PLANS

EXISTING PLANS ARE AVAILABLE UPON REQUEST AT THE MUSKINGUM COUNTY ENGINEER'S OFFICE, 740-454-0155.

WORK LIMITS

THE WORK LIMITS SHOWN ON THESE PLANS ARE FOR PHYSICAL CONSTRUCTION ONLY. PROVIDE THE INSTALLATION AND OPERATION OF ALL WORK ZONE TRAFFIC CONTROL AND WORK ZONE TRAFFIC CONTROL DEVICES REQUIRED BY THESE PLANS WHETHER INSIDE OR OUTSIDE THESE WORK LIMITS.

CLEARING AND GRUBBING

ALTHOUGH THERE ARE NO TREES OR STUMPS SPECIFICALLY MARKED FOR REMOVAL WITHIN THE LIMITS OF THE PROJECT, A LUMP SUM QUANTITY IS INCLUDED IN THE GENERAL SUMMARY FOR ITEM 201, CLEARING AND GRUBBING. ALL PROVISIONS AS SET FORTH IN THE SPECIFICATIONS UNDER THIS ITEM ARE INCLUDED IN THE LUMP SUM PRICE BID FOR ITEM 201, CLEARING AND GRUBBING.

CONTRACTOR'S USE OF RIGHT-OF-WAY

THE CONTRACTOR SHALL NOT USE OR ENTER ANY AREA OUTSIDE OF THE RIGHT-OF-WAY LIMITS THAT ARE SHOWN ON THE PLANS.

FLOOD PLAIN

NO STORAGE OF MATERIALS OR STAGING SHALL OCCURE WITHIN THE FLOODPLAIN OF THOMPSON RUN.

NO IN STREAM WORK

NO WORK SHALL TAKE PLACE BELOW THE ORDINARY HIGH-WATER MARK (OHWM) OF THOMPSON RUN. SHOULD WORK BELOW THE OHWM ELEVATION OF 762.50 NEED TO TAKE PLACE, THE CONTRACTOR WILL BE RESPONSIBLE FOR SECURING THEIR OWN WATERWAY PERMIT FROM THE HUNTINGTON OFFICE OF THE US ARMY CORPS OF ENGINEERS.

DEMOLITION DEBRIS

THE CONTRACTOR SHALL TAKE PRECAUTIONS TO AVOID AND/OR LIMIT DEMOLITION DEBRIS FROM ENTERING THE STREAM. ANY MATERIAL THAT DOES FALL INTO THE STREAM SHALL BE REMOVED WITHIN 72 HOURS.

SEEDING AND MULCHING

THE FOLLOWING QUANTITIES ARE PROVIDED TO PROMOTE GROWTH AND CARE OF PERMANENT SEEDED AREAS:

659, SEEDING AND MULCHING	161 SQ. YD.
659, COMMERCIAL FERTILIZER	0.03 TON
659, LIME	0.1 ACRES
659, WATER	2 M. GAL.

SEEDING AND MULCHING SHALL BE APPLIED TO ALL AREAS OF EXPOSED SOIL BETWEEN THE RIGHT-OF-WAY LINES, AND WITHIN THE CONSTRUCTION LIMITS FOR AREAS OUTSIDE THE RIGHT-OF-WAY LINES COVERED BY WORK AGREEMENT OR SLOPE EASEMENT. QUANTITY CALCULATIONS FOR SEEDING AND MULCHING ARE BASED ON THESE LIMITS.

ENDANGERED BAT HABITAT REMOVAL

THIS PROJECT IS LOCATED WITHIN THE KNOWN HABITAT RANGES OF THE FEDERALLY LISTED AND PROTECTED INDIANA BAT, AND NORTHERN LONG-EARED BAT. NO TREES SHALL BE REMOVED UNDER THIS PROJECT FROM APRIL 1 THROUGH SEPTEMBER 30. ALL NECESSARY TREE REMOVAL SHALL OCCUR FROM OCTOBER 1 THROUGH MARCH 31. THIS REQUIREMENT IS NECESSARY TO AVOID AND MINIMIZE IMPACTS TO THESE SPECIES AS REQUIRED BY THE ENDANGERED SPECIES ACT (ESA). FOR THE PURPOSES OF THIS NOTE, A TREE IS DEFINED AS: A LIVE, DYING, OR DEAD WOODY PLANT, WITH A TRUNK 3 INCHES OR GREATER IN DIAMETER AT A HEIGHT OF 4.5 FEET ABOVE THE GROUND SURFACE, AND WITH A MINIMUM HEIGHT OF 13 FEET.

BENCHING OF FOUNDATION SLOPES

BENCH ALL SLOPED EMBANKMENT AREAS AS SET FORTH IN SECTION 203.05 OF THE CONSTRUCTION AND MATERIAL SPECIFICATIONS (C&MS). NO ADDITIONAL PAYMENT WILL BE MADE FOR BENCHING REQUIRED UNDER THE PROVISIONS OF SECTION 203.05

OEPA NOTIFICATION OF DEMOLITION AND RENOVATION

AN ASBESTOS SURVEY FOR THE MUS-TR90-4.00 BRIDGE SCHEDULED FOR DEMOLITION WORK WAS CONDUCTED BY A CERTIFIED ASBESTOS HAZARD EVALUATION SPECIALIST. A COPY OF THE ASBESTOS SURVEY REPORT FOR THE BRIDGE HAS BEEN INCLUDED IN THE PLAN PACKAGE FOR THIS PROJECT. THE ASBESTOS SURVEY REPORT DID NOT IDENTIFY THE PRESENCE OF ANY ASBESTOS CONTAINING MATERIALS.

A COPY OF THE OHIO ENVIRONMENTAL PROTECTION AGENCY (OEPA) NOTIFICATION OF DEMOLITION AND RENOVATION FORM, PARTIALLY COMPLETED BY THE ASBESTOS HAZARD EVALUATION SPECIALIST, HAS BEEN INCLUDED AT THE END OF THE ASBESTOS SURVEY REPORT. THE CONTRACTOR SHALL COMPLETE THE NECESSARY SECTIONS OF THE FORM AND SUBMIT IT WITH A COPY OF THE ASBESTOS SURVEY REPORT TO:

ASBESTOS PROGRAM
OHIO EPA, DAPC
PO BOX 1049
COLUMBUS OH 43216-1049

AT LEAST 10 WORKING DAYS PRIOR TO THE START OF ANY DEMOLITION WORK. NOTIFICATION CAN BE MADE EITHER BY HARD COPY OR ELECTRONICALLY. ADDITIONAL INFORMATION CAN BE FOUND HERE:
<http://epa.ohio.gov/dapc/atu/asbestos.aspx#179575188-project-notification>

BASIS FOR PAYMENT:

THE CONTRACTOR SHALL FURNISH ALL FEES, LABOR, AND MATERIAL NECESSARY TO COMPLETE AND SUBMIT THE OEPA NOTIFICATION FORM. PAYMENTS FOR THIS WORK SHALL BE INCIDENTAL TO THE ITEM 202 STRUCTURE REMOVAL ITEM(S) IN THE PLAN.

GENERAL NOTES

DESIGN AGENCY
Muskingum
COUNTY
ENGINEER'S
OFFICE

DESIGNER
SEJ

REVIEWER
GJW 3/20/23

PROJECT ID
117331

SHEET TOTAL
3 18

ITEM 614 - MAINTAINING TRAFFIC

THE CONTRACTOR SHALL PROVIDE, ERECT AND MAINTAIN STANDARD 48"x30" ROAD CLOSED SIGNS, ADVANCED WARNING SIGNS, SIGN SUPPORTS, BARRICADES, GATES AND LIGHTS AS SHOWN ON SCD MT-101.60 AT THE LOCATIONS SHOWN DURING PERIODS IN WHICH THE AFFECTED ROADS ARE CLOSED TO TRAFFIC.

THE CONTRACTOR SHALL PROVIDE, ERECT AND MAINTAIN SIGNS, SIGN SUPPORTS, AND TYPE 3 BARRICADES OF THE TYPE AND LOCATION AS SHOWN ON THIS SHEET.

ALL WORK AND TRAFFIC DEVICES SHALL BE IN ACCORDANCE WITH CMS 614 AND OTHER APPLICABLE PORTIONS OF THE SPECIFICATIONS, AS WELL AS THE OHIO MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES.

DETOUR NOTIFICATION

THE CONTRACTOR SHALL ADVISE THE MCEO EIGHTEEN (18) DAYS IN ADVANCE OF WHEN THE DETOUR ROUTE WILL BE IN EFFECT. THE CONTRACTOR SHALL THEN PROVIDE AND INSTALL ALL DEVICES NECESSARY TO DEFINE THE ROUTE OF THE DETOUR AND SHALL MAINTAIN THE SAME THROUGHOUT THE DETOUR LIMITATION DATES. ALL TRAFFIC CONTROL DEVICES REQUIRED, SHALL BE FURNISHED, ERECTED, MAINTAINED, AND SUBSEQUENTLY REMOVED BY THE CONTRACTOR.

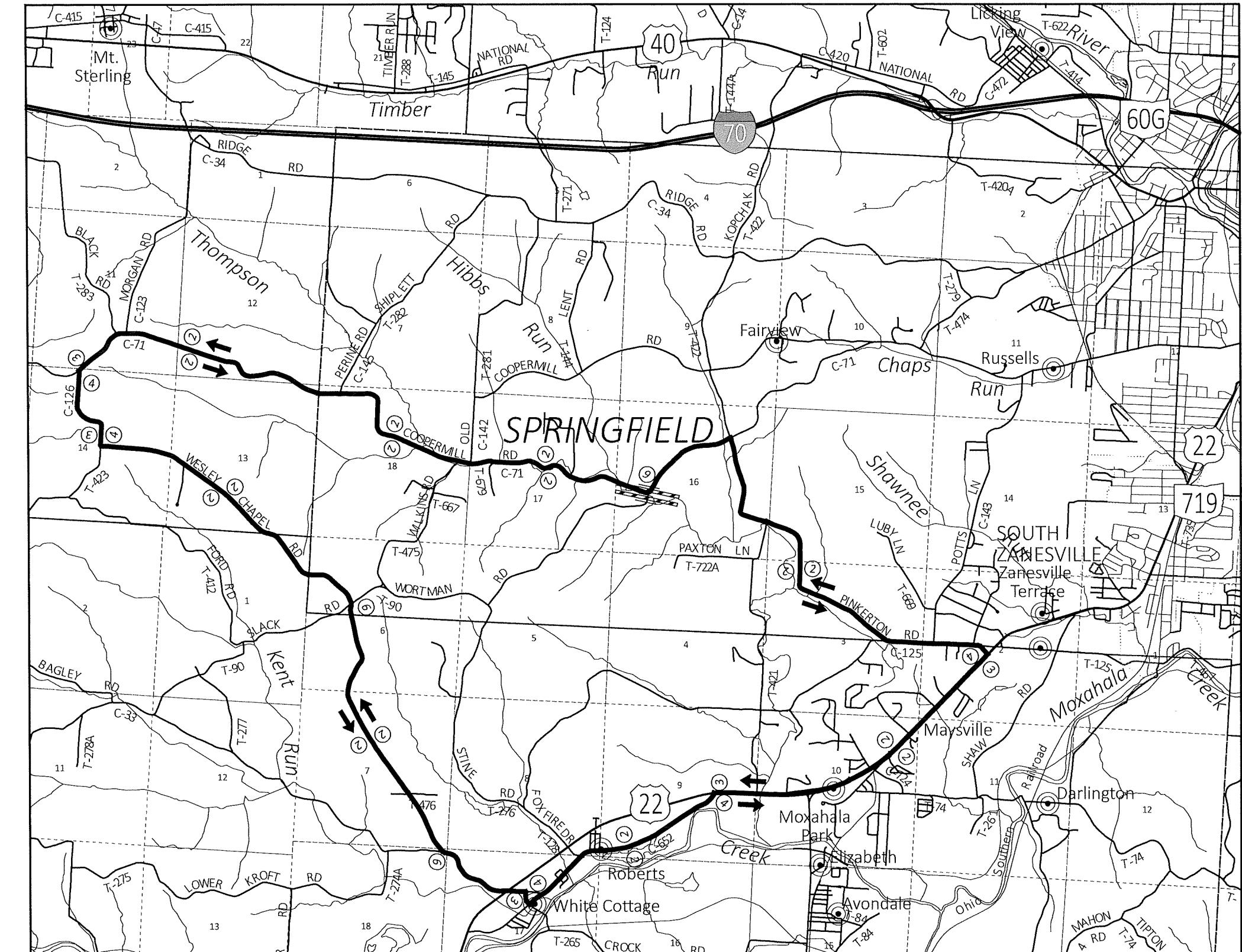
DETOUR SIGNAGE

THE CONTRACTOR SHALL ERECT AND MAINTAIN DETOUR SIGNAGE AND ADVANCED NOTICE SIGNS AS SHOWN ON THIS SHEET.

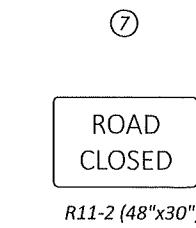
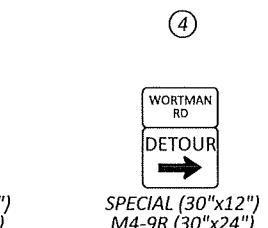
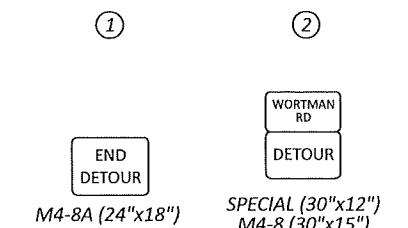
THIS WORK SHALL BE PAID UNDER THE LUMP SUM PAY ITEM 614 - DETOUR SIGNING, AS PER PLAN

DETOUR LIMITATION

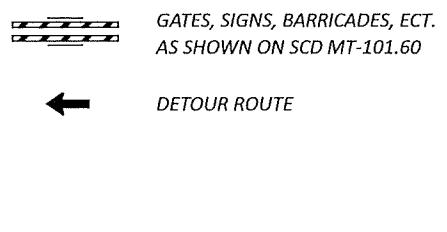
THE MAXIMUM LENGTH OF TIME FOR THE DETOUR ROUTE TO BE IN EFFECT SHALL BE NINETY DAYS (90) CONSECUTIVE DAYS. CONSTRUCTION WORK MAY BE PERFORMED BEFORE AND AFTER THE DETOUR LIMITATION DATES, BUT THERE SHALL BE NO RESTRICTIONS TO THROUGH OR LOCAL TRAFFIC. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO SCHEDULE AND PERFORM THE CONSTRUCTION WORK WITHIN THE DETOUR LIMITATION TIME. THE FAILURE OF THE CONTRACTOR TO MEET THE DETOUR LIMITATION DATES WILL CAUSE SEPARATE LIQUIDATED DAMAGES IN ACCORDANCE WITH 108.07 TO BE ASSESSED. THE CONTRACTOR SHALL COMPLY WITH ALL PROVISIONS OF 108.07 OF THE DEPARTMENT OF TRANSPORTATION CONSTRUCTION AND MATERIAL SPECIFICATIONS.



TYPICAL SIGNS AS NEEDED



LEGEND



DESIGNER
SEJ
REVIEWER
GJW 3/20/23
PROJECT ID
117331
SHEET 4
TOTAL 18

SHEET NUM.			PART.	ITEM	ITEM EXT	GRAND TOTAL	UNIT	DESCRIPTION	SEE SHEET NO.
ROADWAY									
				201	11000	LS		CLEARING AND GRUBBING	
				202	75000	110	FT	FENCE REMOVED	
				203	10000	111	CY	EXCAVATION	
				203	20000	74	CY	EMBANKMENT	
				204	10000	422	SY	SUBGRADE COMPACTION	
				606	15050	187.5	FT	GUARDRAIL, TYPE MGS	
				606	25550	2	EACH	ANCHOR ASSEMBLY, MGS TYPE A	
				606	26550	2	EACH	ANCHOR ASSEMBLY, MGS TYPE T	
EROSION CONTROL									
				601	32204	25	CY	ROCK CHANNEL PROTECTION, TYPE C WITH GEOTEXTILE FABRIC	
				659	00530	161	SY	SEEDING AND MULCHING, CLASS 3B	
				659	20000	0.03	TON	COMMERCIAL FERTILIZER	
				659	31000	0.1	ACRE	LIME	
				659	35000	2	MGAL	WATER	
				832	30000	2,000	EACH	EROSION CONTROL	
PAVEMENT									
				301	46000	38	CY	ASPHALT CONCRETE BASE, PG64-22	
				304	20000	58	CY	AGGREGATE BASE	
				407	10000	17	GAL	TACK COAT	
				411	10000	14	CY	STABILIZED CRUSHED AGGREGATE	
				441	50000	12	CY	ASPHALT CONCRETE SURFACE COURSE, TYPE 1, (448), PG64-22	
				441	50300	17	CY	ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 2, (448)	
TRAFFIC CONTROL									
				626	00110	8	EACH	BARRIER REFLECTOR, TYPE 2, BI-DIRECTIONAL	
				642	00300	0.05	MILE	CENTER LINE, TYPE 1	
MAINTENANCE OF TRAFFIC									
				614	12421	LS		DETOUR SIGNING, AS PER PLAN	4
STRUCTURE OVER 20 FOOT SPAN (MUS-TR90-4.00)									
FOR BRIDGE ESTIMATED QUANTITIES SEE SHEET 11									
INCIDENTALS									
				614	11000	LS		MAINTAINING TRAFFIC	
				623	10000	LS		CONSTRUCTION LAYOUT STAKES AND SURVEYING	
				624	10000	LS		MOBILIZATION	
MUS-T.R.90-4.00	MODEL: Sheet1111 (In.) DATE: 3/23/2023 TIME: 2:24:00 PM USER: sejohnson DIMCCEQ Project ID: 117331400-Engineering>Roadway>Sheets117331_Geo001.dgn	DESIGN AGENCY Rising Sun COUNTY'S OFFICE ENGINEER'S OFFICE	DESIGNER SEJ REVIEWER GJW 3/20/23 PROJECT ID 117331 SHEET TOTAL 5 18	SEE SHEET NO.	GENERAL SUMMARY				

NOTE:
EXISTING TRUSS BRIDGE (16' WIDE X 68' SPAN) TO BE REPLACED
WITH PROPOSED (20' WIDE X 61' SPAN) CONCRETE BOX BEAM BRIDGE.

2

BEGIN PROJECT STA. 0+50.00 ELEV. = 769.83 Δ = 0.00% NO CURVE

21.75' V.C. P.V.I. STA. 1+29.12 ELEV. 770.50 SSD = 408'

41.00' V.C. P.V.I. STA. 0+78.88 ELEV. 769.13 SSD = 90'

END PROJECT STA. 2+83.00 ELEV. = 767.28 Δ = 0.00% NO CURVE

BRIDGE LIMITS = 64.00' SPAN = 61'-0" C/C BRGS.

OHWM ELEV. = 762.50±

EXISTING ABUTMENT TO BE REMOVED TO ELEVATION 765.00'

PROPOSED H-PILE (TYP.)

EXISTING ABUTMENT TO BE REMOVED TO ELEVATION 765.00'

VPI 0+50.00 Elev. 769.83

P.V.I. STA. 1+29.12 ELEV. 770.50 SSD = 408'

P.V.I. STA. 0+78.88 ELEV. 769.13 SSD = 90'

P.V.I. STA. 2+62.10 ELEV. 767.59 SSD = 92'

END PROJECT STA. 2+83.00 ELEV. = 767.28 Δ = 0.00% NO CURVE

BRIDGE LIMITS = 64.00' SPAN = 61'-0" C/C BRGS.

OHWM ELEV. = 762.50±

EXISTING ABUTMENT TO BE REMOVED TO ELEVATION 765.00'

PROPOSED H-PILE (TYP.)

VPI 2+83.00 Elev. 767.28

END PROJECT STA. 2+83.00 ELEV. = 767.28 Δ = 0.00% NO CURVE

BRIDGE LIMITS = 64.00' SPAN = 61'-0" C/C BRGS.

OHWM ELEV. = 762.50±

EXISTING ABUTMENT TO BE REMOVED TO ELEVATION 765.00'

PROPOSED H-PILE (TYP.)

VPI 2+83.00 Elev. 767.28

**PLAN AND PROFILE
STA. 0+45 TO STA. 3+19**

The seal is circular with a decorative border. The words "CITY OF KING COUNTY" are written in a circular pattern around the border. In the center, the words "WASHINGTON" are written vertically, and "CITY OF KING COUNTY" is written horizontally across the center.

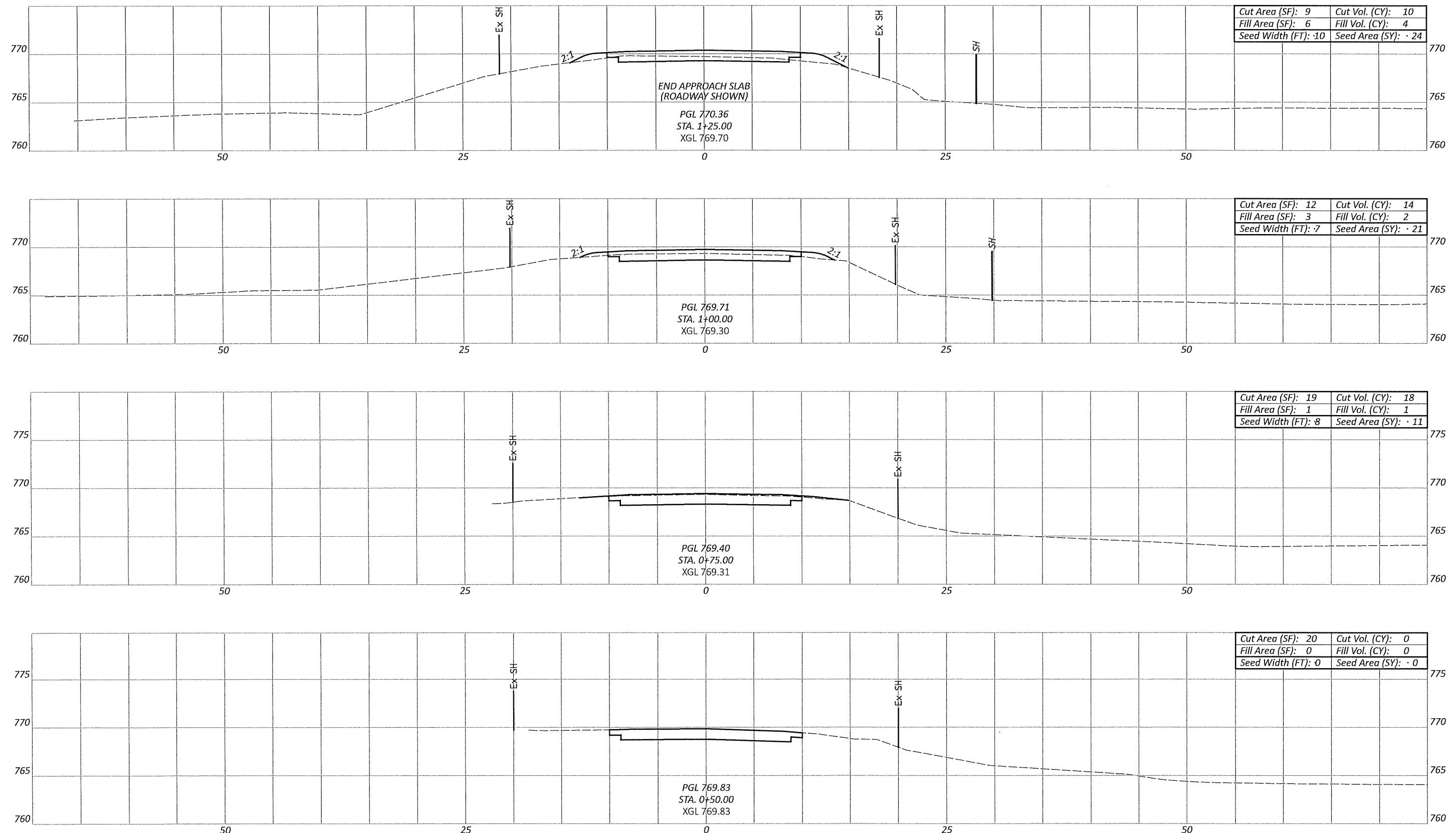
SEJ
REVIEWER

JW 3/20/2018

117331

6 | 18

ROADWAY CROSS SECTIONS
STA. 0+50.00 TO 1+25.00



DESIGN AGENCY
WASHINGTON COUNTY
ENGINEER'S OFFICE

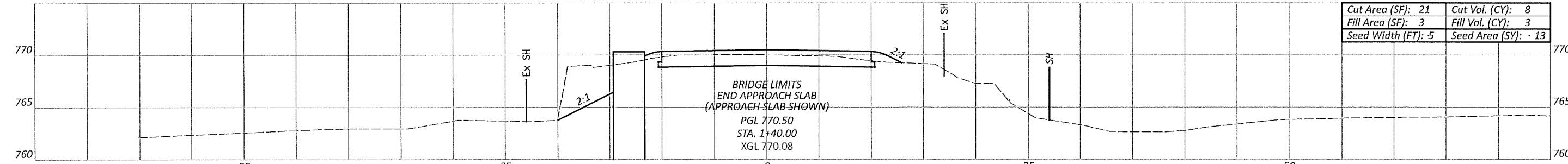
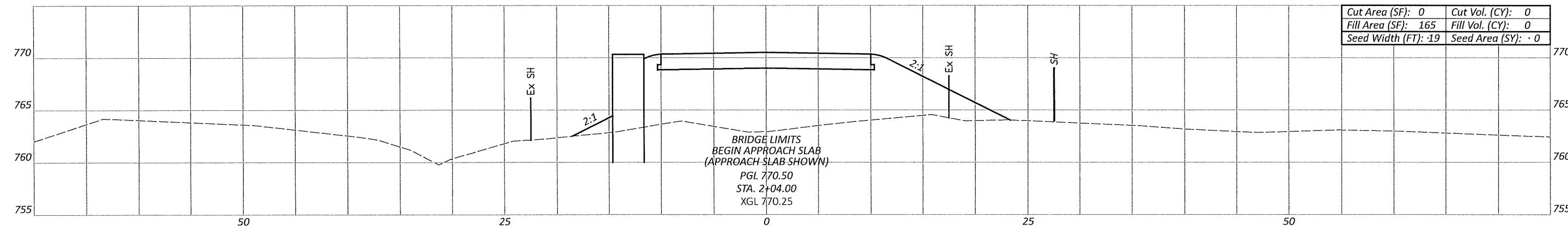
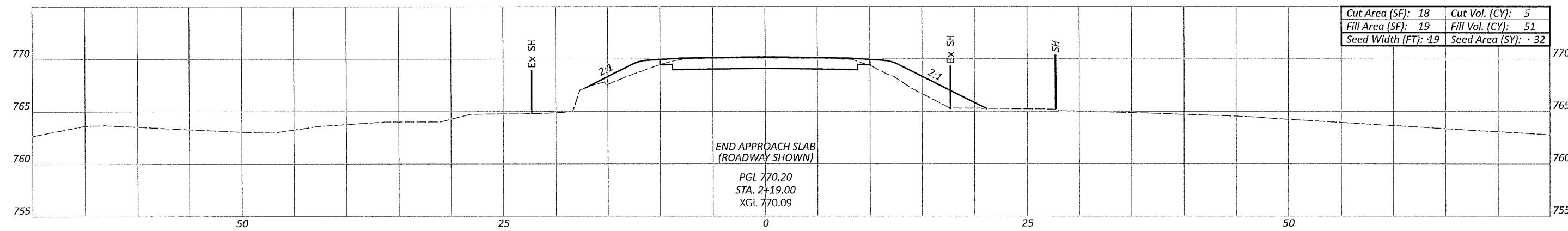
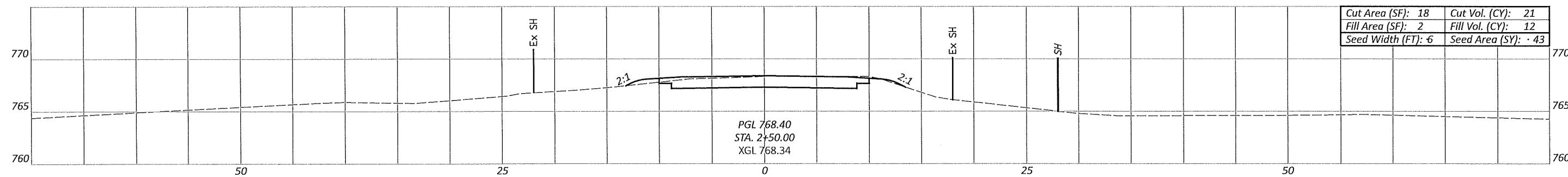
DESIGNER
SEJ

REVIEWER
GJW 3/20/23

PROJECT ID
117331

Sheet Totals
Seeding Cut Fill
56 42 7

SHEET TOTAL
7 18



ROADWAY CROSS SECTIONS STA. 1+40.00 TO 2+50.00

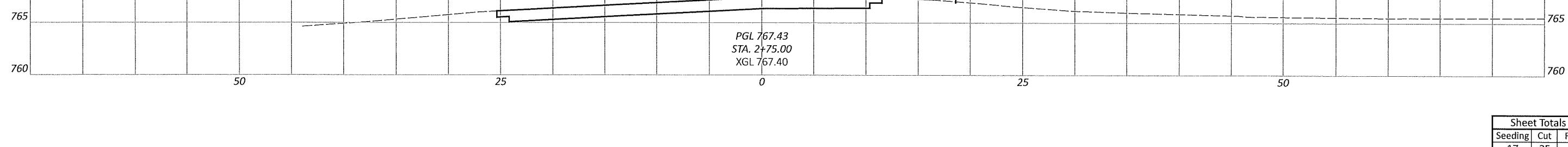
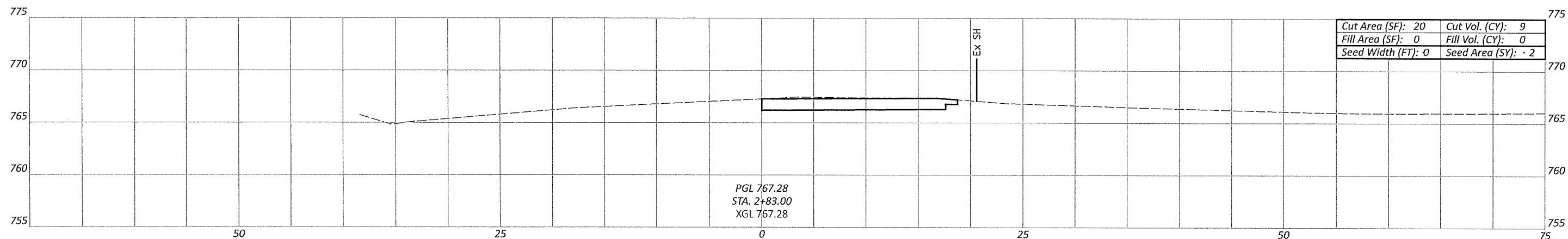


DESIGNER
SEJ
REVIEWER
GJW 3/20/23

PROJECT ID
117331

Sheet Totals
Seeding Cut Fill
88 34 66

Sheet Totals
Seeding Cut Fill
8 18



STANDARD DRAWINGS AND SUPPLEMENTAL SPECIFICATIONS

REFER TO THE FOLLOWING STANDARD BRIDGE DRAWING(S):

AS-1-15 DATED (REVISED) 7/17/15
 DS-1-92 DATED (REVISED) 7/15/22
 PSBD-2-07 DATED (REVISED) 7/20/18

AND TO THE FOLLOWING SUPPLEMENTAL SPECIFICATION(S):

800 DATED 4/21/23
 832 DATED 7/15/22

DESIGN SPECIFICATIONS

THIS STRUCTURE CONFORMS TO THE 9th EDITION OF THE "LRFD BRIDGE DESIGN SPECIFICATIONS" ADOPTED BY THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS, AND THE ODOT BRIDGE DESIGN MANUAL, 2020.

OPERATIONAL IMPORTANCE

A LOAD MODIFIER OF 1.0 HAS BEEN ASSUMED FOR THE DESIGN OF THIS STRUCTURE IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, ARTICLE 1.3.5 AND THE ODOT BRIDGE DESIGN MANUAL.

DESIGN LOADING

DESIGN LOADING INCLUDES:

VEHICULAR LIVE LOAD: HL-93
 FUTURE WEARING SURFACE (FWS) OF 0.060 KIPS/SQ.FT

DESIGN DATA

CONCRETE CLASS QC2:

COMPRESSIVE STRENGTH 4.5 KSI (SUPERSTRUCTURE)

CONCRETE CLASS QC1:

COMPRESSIVE STRENGTH 4.0 KSI (SUBSTRUCTURE)

REINFORCING STEEL MINIMUM YIELD STRENGTH 60 KSI

STEEL H-PILES - ASTM A572: YIELD STRENGTH 50 KSI

CONCRETE FOR PRESTRESSED BEAMS:

COMPRESSIVE STRENGTH (FINAL) - 7.0 KSI

COMPRESSIVE STRENGTH (RELEASE) - 5.0 KSI

PRE-STRESSING STRAND:

AREA = 0.153 SQUARE INCHES

ULTIMATE STRENGTH = 270 KSI

INITIAL STRESS = 202.5 KSI (LOW RELAXATION STRANDS)

EXISTING STRUCTURE VERIFICATION

DETAILS AND DIMENSIONS SHOWN ON THESE PLANS PERTAINING TO THE EXISTING STRUCTURE HAVE BEEN OBTAINED FROM PLANS OF THE EXISTING STRUCTURE AND FROM FIELD OBSERVATIONS AND MEASUREMENTS. CONSEQUENTLY, THEY ARE INDICATIVE OF THE EXISTING STRUCTURE AND THE PROPOSED WORK BUT THEY SHALL BE CONSIDERED TENTATIVE AND APPROXIMATE. THE CONTRACTOR IS REFERRED TO C&MS, SECTIONS 102.05, 105.02, AND 513.04. BASE CONTRACT BID PRICES UPON A RECOGNITION OF THE UNCERTAINTIES DESCRIBED ABOVE AND UPON A PREBID EXAMINATION OF THE EXISTING STRUCTURE. HOWEVER, THE DEPARTMENT WILL PAY FOR ALL PROJECT WORK BASED UPON ACTUAL DETAILS AND DIMENSIONS THAT HAVE BEEN VERIFIED IN THE FIELD.

ESTIMATED QUANTITIES										SPEC & AS PER PLAN BRIDGE SHEET NO.		
ITEM	ITEM EXT.	TOTAL	UNIT	DESCRIPTION					ABUTS.	SUPER	GEN'L	
202	11203	LS		PORTIONS OF STRUCTURE REMOVED, OVER 20 FOOT SPAN, AS PER PLAN								LS [2/9]
202	23500	126	SY	WEARING COURSE REMOVED								126
203	35110	55	CY	GRANULAR MATERIAL, TYPE B								55
503	11100	LS		COFFERDAMS AND EXCAVATION BRACING								LS
503	21301	LS		UNCLASSIFIED EXCAVATION, AS PER PLAN								LS [2/9]
505	11100	LS		PILE DRIVING EQUIPMENT MOBILIZATION								LS
507	00200	270	FT	STEEL PILES HP12X53, FURNISHED								270
507	00250	210	FT	STEEL PILES HP12X53, DRIVEN								210
509	10000	8954	LB	EPOXY COATED REINFORCING STEEL								4266 4688
511	21533	28	CY	CLASS QC2 CONCRETE, SUPERSTRUCTURE, AS PER PLAN								28 [2/9]
511	45511	156	CY	CLASS QC1 CONCRETE, ABUTMENT								156
512	10100	80	SY	SEALING OF CONCRETE SURFACES (EPOXY-URETHANE)								43 37
515	12051	5	EACH	PRESTRESSED CONCRETE COMPOSITE BOX BEAM MEMBERS, LEVEL 1, CB21-48, AS PER PLAN								LS [2/9]
516	13600	15	SF	1" PREFORMED EXPANSION JOINT FILLER								15
516	14002	60	FT	SEMI-INTEGRAL ABUTMENT EXPANSION JOINT SEAL								60
516	41100	40	EACH	1/8" PREFORMED BEARING PADS								40
516	44000	20	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES ONLY (NEOPRENE), 6"x12"x1.5"								20
526	10000	68	SY	REINFORCED CONCRETE APPROACH SLAB (T=12")								68
606	98000	112.5	FT	GUARDRAIL, MISC.: RAILING SYSTEM SIDE MOUNTED MGS								112.5 [2/9]
SPECIAL	51822300	116	FT	STEEL DRIP STRIP								116

MONOLITHIC WEARING SURFACE

MONOLITHIC WEARING SURFACE IS ASSUMED, FOR DESIGN PURPOSES, TO BE 1 INCH THICK.

ITEM 503 - UNCLASSIFIED EXCAVATION, AS PER PLAN

THE BACKFILL MATERIAL SHALL BE GRANULAR MATERIAL, TYPE B AS SHOWN ON SHEETS [4/9] AND [6/9]. THIS COST SHALL BE INCLUDED IN THIS PAY ITEM.

ITEM 511 - CLASS QC2 CONCRETE, SUPERSTRUCTURE, AS PER PLAN

THE CONTRACTOR SHALL ENSURE THAT IPANEX WATERPROOFING, OR APPROVED EQUAL, IS ADDED TO THE CONCRETE MIXTURE AS REQUIRED BY THE ENGINEER. STANDARD CLASS QC2 CONCRETE SHALL BE USED WITH THE ADDITION OF IPANEX WATERPROOFING AT A RATE OF 13.8 OZ PER 100 LB OF CEMENT OR CEMENTATION MATERIAL. THIS COST SHALL BE INCLUDED IN THIS PAY ITEM.

ITEM 202, PORTIONS OF STRUCTURE REMOVED, OVER 20 FOOT SPAN, AS PER PLAN

THE EXISTING ABUTMENTS SHALL BE REMOVED TO ELEVATION 765.00. THE STEEL SUPERSTRUCTURE MEMBERS SHALL BE REMOVED FOR SALVAGE AND BECOME PROPERTY OF THE LANDOWNER. THE CONTRACTOR SHALL MOVE THE SALVAGED STEEL TO A LOCATION DESIGNATED BY THE LANDOWNER ON THE SAME PARCEL.

THE DECK SHALL BE CAREFULLY REMOVED ABOVE THE STRINGERS AND FLOOR BEAMS, THEN DISPOSED OF BY THE CONTRACTOR. THE STRINGERS AND FLOOR BEAMS SHALL BE CAREFULLY CUT AS CLOSE TO THE CONNECTION POINTS AS POSSIBLE AND HAULED AWAY. EACH STEEL TRUSS SHALL BE REMOVED AND HAULED AWAY IN ONE PIECE.

PRIOR TO THE DEMOLITION, THE CONTRACTOR SHALL ESTABLISH A RIGHT-OF-ENTRY AGREEMENT WITH THE LANDOWNER TO RELOCATE THE SALVAGED STEEL.

PILE TO BEDROCK

DRIVE PILES TO REFUSAL ON BEDROCK. THE COUNTY WILL WILL CONSIDER REFUSAL TO BE OBTAINED WHEN THE PILE PENETRATION IS AN INCH OR LESS AFTER RECEIVING AT LEAST 20 BLOWS FROM THE PILE HAMMER. SELECT THE HAMMER SIZE TO ACHIEVE THE REQUIRED DEPTH TO BEDROCK AND REFUSAL.

THE TOTAL FACTORED LOAD IS 45 KIPS PER PILE FOR THE HP12X53 ABUTMENT PILES.

ABUTMENT PILES:

6 PILES 25 FEET LONG, ORDER LENGTH AT REAR ABUTMENT
 6 PILES 20 FEET LONG, ORDER LENGTH AT FORWARD ABUTMENT

BEARING PAD SHIMS

PLACE 1/8" THICK PREFORMED BEARING PAD SHIMS, PLAN AREA 6 INCHES BY 10 INCHES, UNDER THE ELASTOMERIC BEARING PADS WHERE REQUIRED FOR PROPER BEARING. FURNISH TWO SHIMS PER BEAM. THE COUNTY WILL MEASURE THIS ITEM BY THE TOTAL NUMBER SUPPLIED. THE COUNTY WILL PAY FOR ACCEPTED QUANTITIES AT THE CONTRACT PRICE FOR ITEM 516 - 1/8" PREFORMED BEARING PADS. ANY UNUSED SHIMS BECOME THE PROPERTY OF THE COUNTY

ITEM 515 - PRESTRESSED CONCRETE COMPOSITE BOX BEAM MEMBERS, LEVEL 1, CB21-48, AS PER PLAN

PRIOR TO FABRICATION, THE CONTRACTOR SHALL SUBMIT TO THE COUNTY A LOAD RATING REPORT WITH BR100 PER THE LATEST ODOT BRIDGE DESIGN MANUAL.

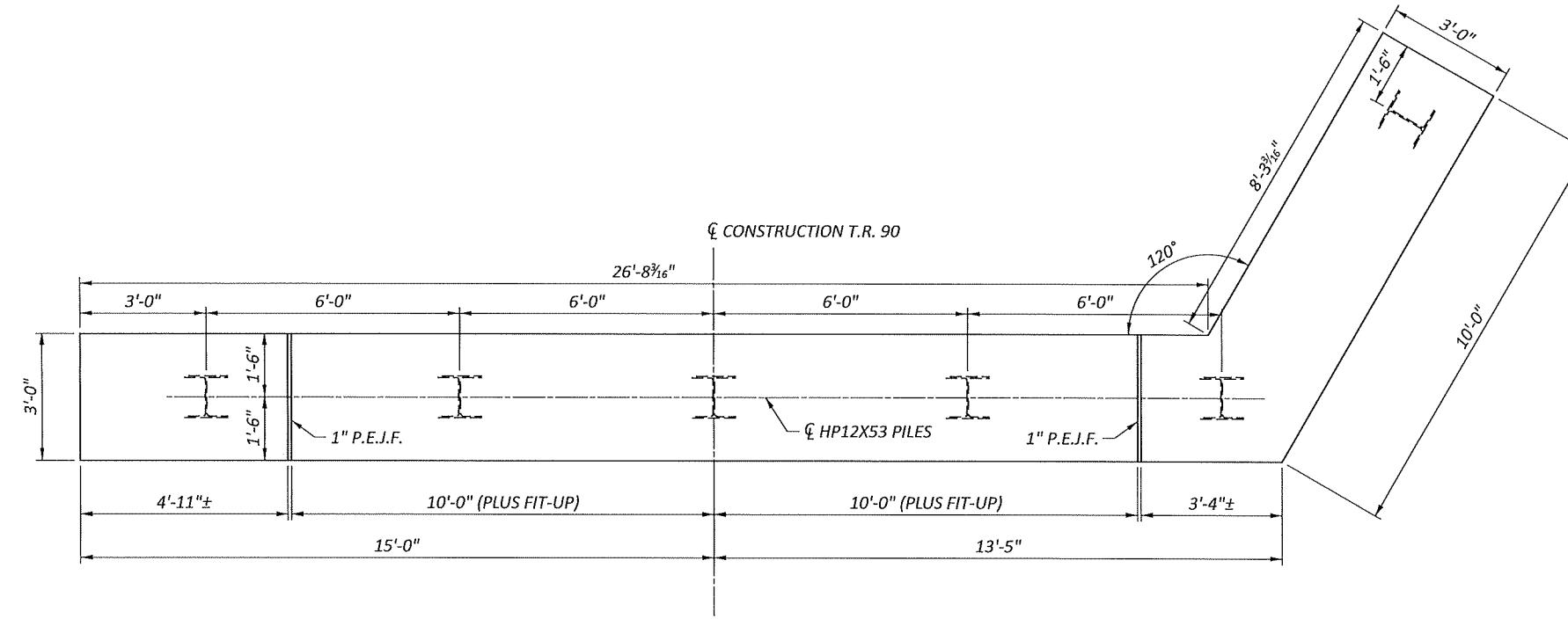
THE LOAD RATING REPORT SHALL ALSO INCLUDE THE CALCULATED CAMBER AT RELEASE AND TIME OF DECK POUR. LONG TERM CAMBER AND TOTAL CALCULATED DEAD LOAD DEFLECTION SHALL BE PROVIDED AS WELL IN ORDER FOR THE CONTRACTOR TO ESTABLISH SCREED ELEVATIONS AND ADJUST BEAM SEAT ELEVATIONS IF NEEDED.

ITEM 606 - GUARDRAIL, MISC.: RAILING SYSTEM SIDE MOUNTED MGS

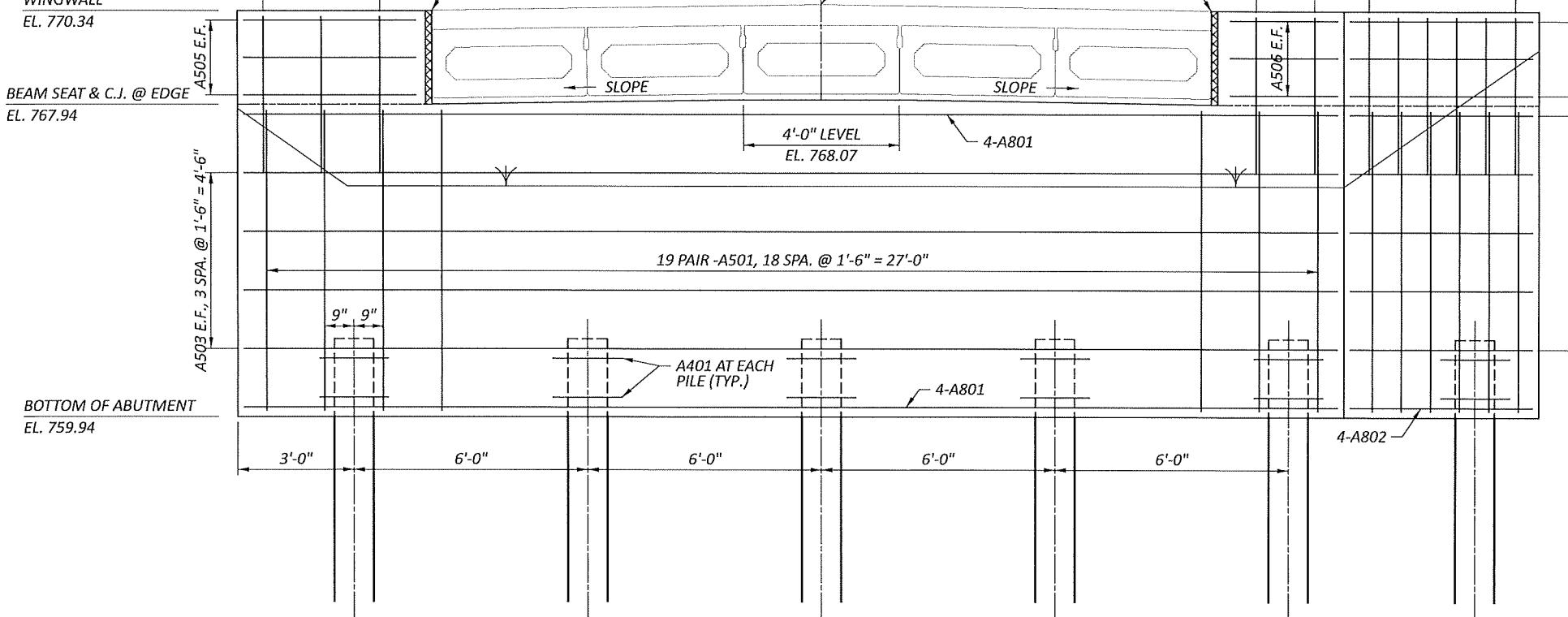
THE CONTRACTOR SHALL REFER TO THE PLAN INSERTION SHEET "RAILING SYSTEM SIDE MOUNTED MGS" INCLUDED IN THE PLANS FOR DETAILS ON THE BRIDGE RAIL SYSTEM.

SHEET NO.

1



ABUTMENT PLAN VIEW



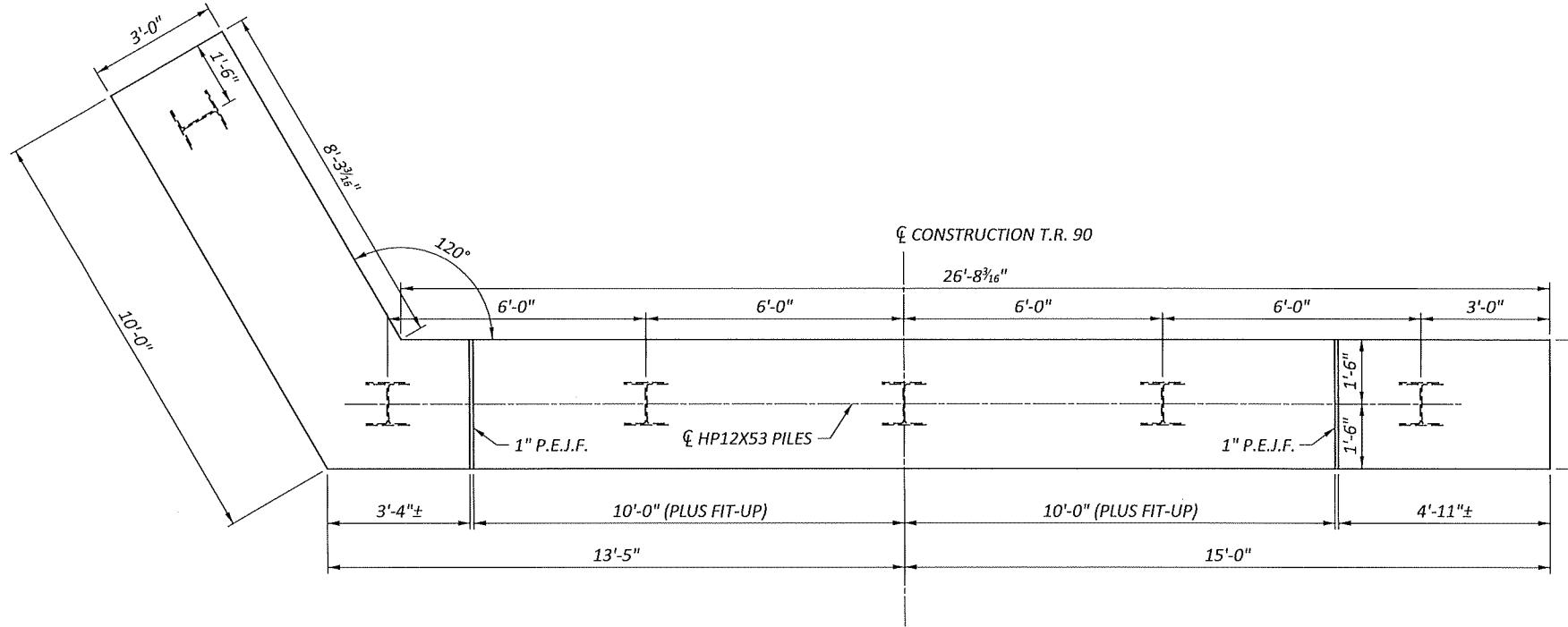
ABUTMENT ELEVATION VIEW

NOTES:

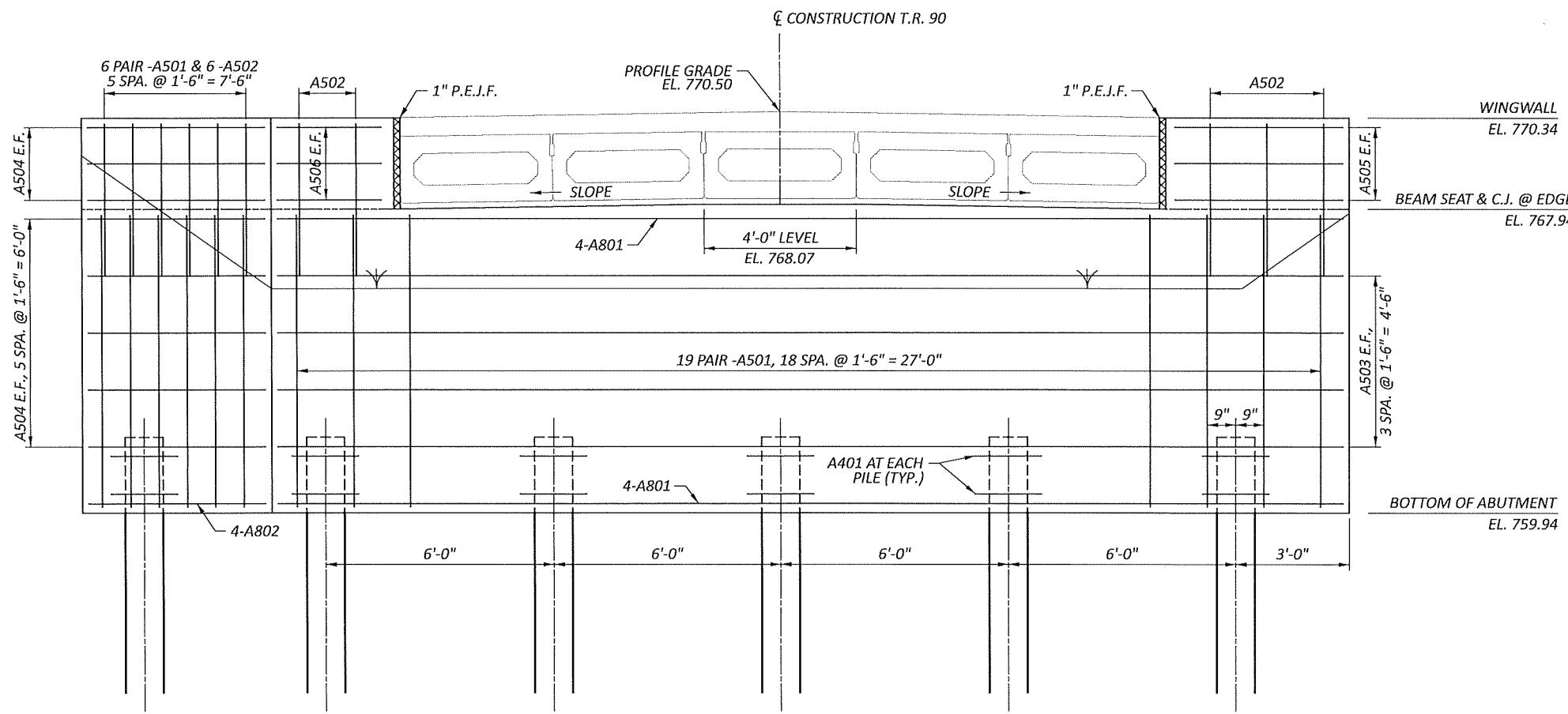
1. S501, S502, S503, S801 BARS NOT SHOWN,
SEE BRIDGE SHEET 4/9

REAR ABUTMENT DETAILS
BRIDGE NO. MUS-TR90-0400
OVER THOMPSON RUN

SFN	6038396
DESIGN AGENCY	
DESIGNER	CHECKER
SEJ	GJW
REVIEWER	
GJW	3/20/23
PROJECT ID	117331
SUBSET	TOTAL
3	9
SHEET	TOTAL
12	18



ABUTMENT PLAN VIE



ABUTMENT ELEVATION VII

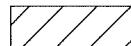
NOTES:

1. S501, S502, S503, S801 BARS NOT SHOWN,
SEE BRIDGE SHEET 6/9

SFN 6038396	
DESIGN AGENCY	
 MASSACHUSETTS COMMONWEALTH ENGINEER'S OFFICE	
DESIGNER SEJ	CHECKER GJW
REVIEWER GJW 3/20/23	
PROJECT ID 117331	
SUBSET 5	TOTAL 9
SHEET 14	TOTAL 18

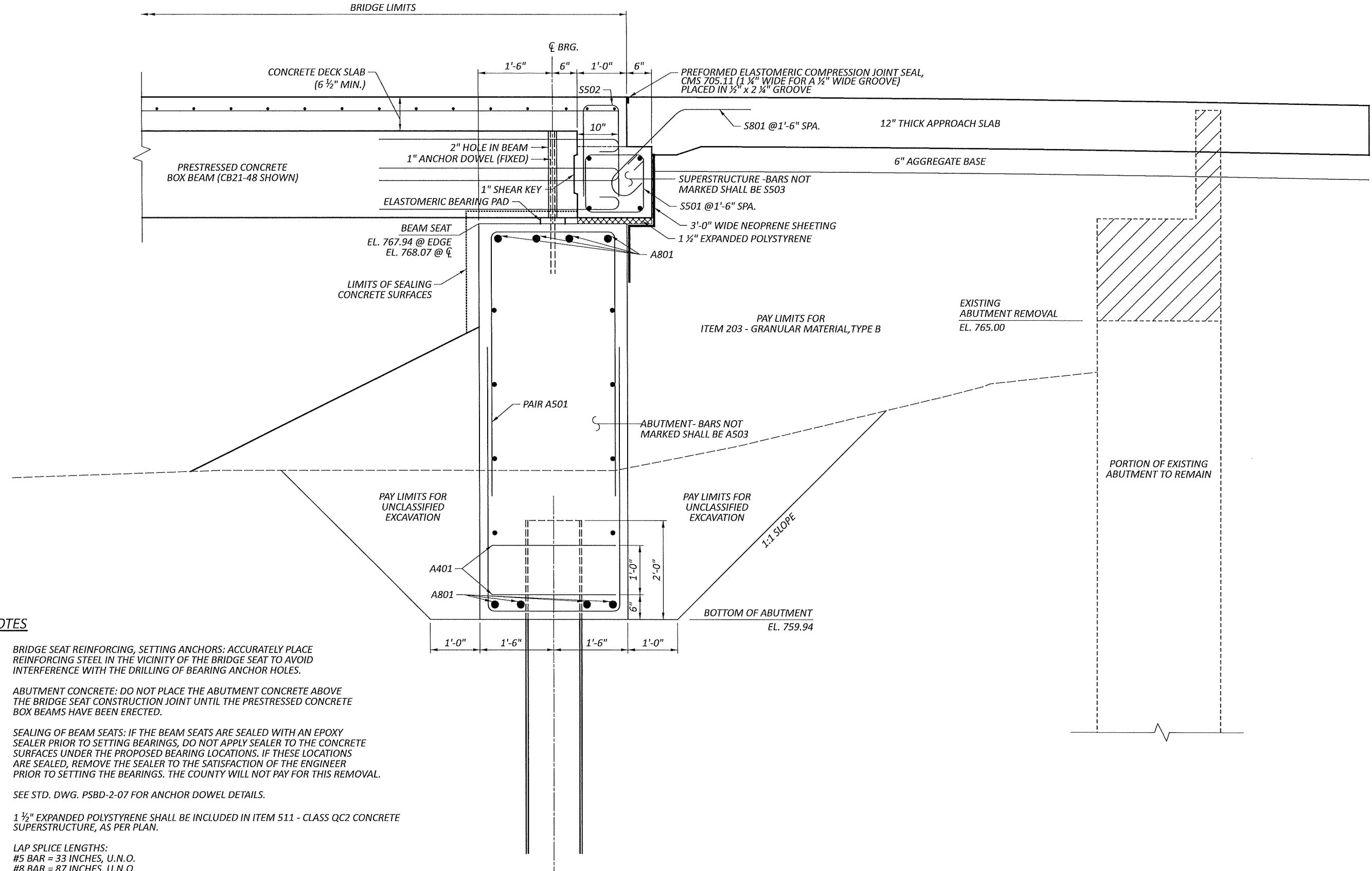
1. BRIDGE SEAT REINFORCING, SETTING ANCHORS: ACCURATELY PLACE REINFORCING STEEL IN THE VICINITY OF THE BRIDGE SEAT TO AVOID INTERFERENCE WITH THE DRILLING OF BEARING ANCHOR HOLES.
2. ABUTMENT CONCRETE: DO NOT PLACE THE ABUTMENT CONCRETE ABOVE THE BRIDGE SEAT CONSTRUCTION JOINT UNTIL THE PRESTRESSED CONCRETE BOX BEAMS HAVE BEEN ERECTED.
3. SEALING OF BEAM SEATS: IF THE BEAM SEATS ARE SEALED WITH AN EPOXY SEALER PRIOR TO SETTING BEARINGS, DO NOT APPLY SEALER TO THE CONCRETE SURFACES UNDER THE PROPOSED BEARING LOCATIONS. IF THESE LOCATIONS ARE SEALED, REMOVE THE SEALER TO THE SATISFACTION OF THE ENGINEER PRIOR TO SETTING THE BEARINGS. THE COUNTY WILL NOT PAY FOR THIS REMOVAL.
4. SEE STD. DWG. PSBD-2-07 FOR ANCHOR DOWEL DETAILS.
5. 1 1/2" EXPANDED POLYSTYRENE SHALL BE INCLUDED IN ITEM 511 - CLASS QC2 CONCRETE SUPERSTRUCTURE, AS PER PLAN.
6. LAP SPLICE LENGTHS:
#5 BAR = 33 INCHES, U.N.O.
#8 BAR = 87 INCHES, U.N.O.

LEGEND



EXISTING ABUTMENT REMOVAL LIMITS

ABUTMENT SECTION



FORWARD ABUTMENT DETAILS
BRIDGE NO. MUS-TR90-0400
OVER THOMPSON RUN

IGNER CHECK
SEJ GJW

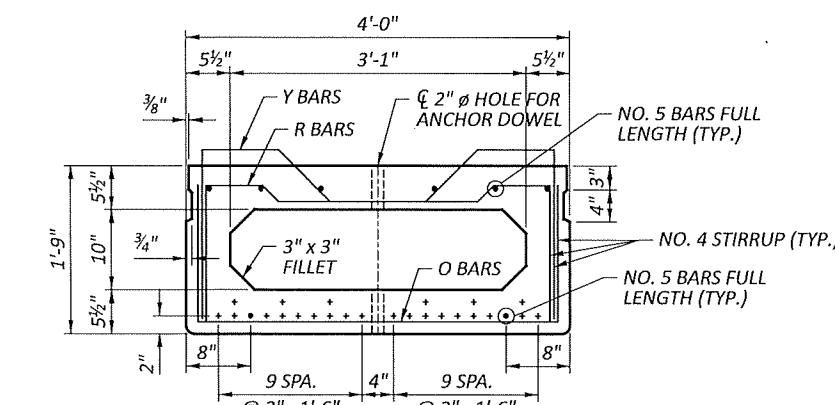
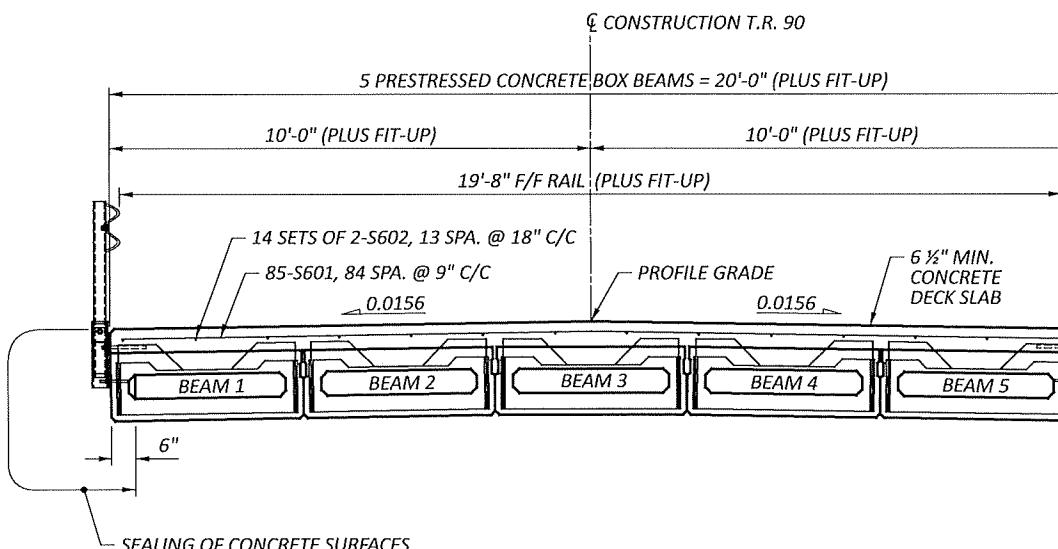
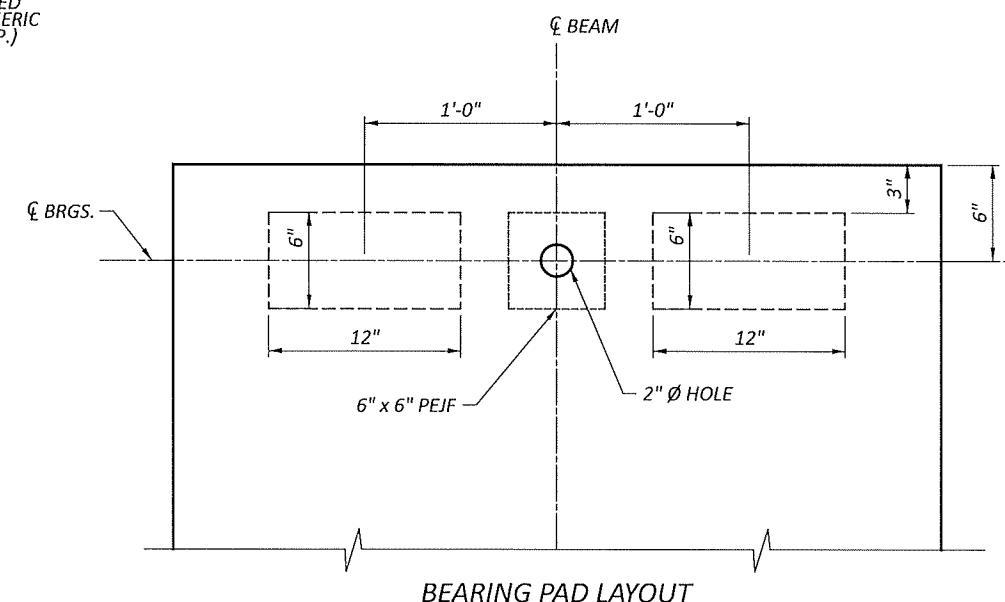
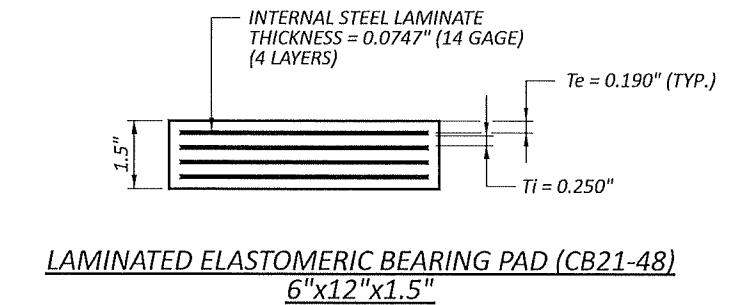
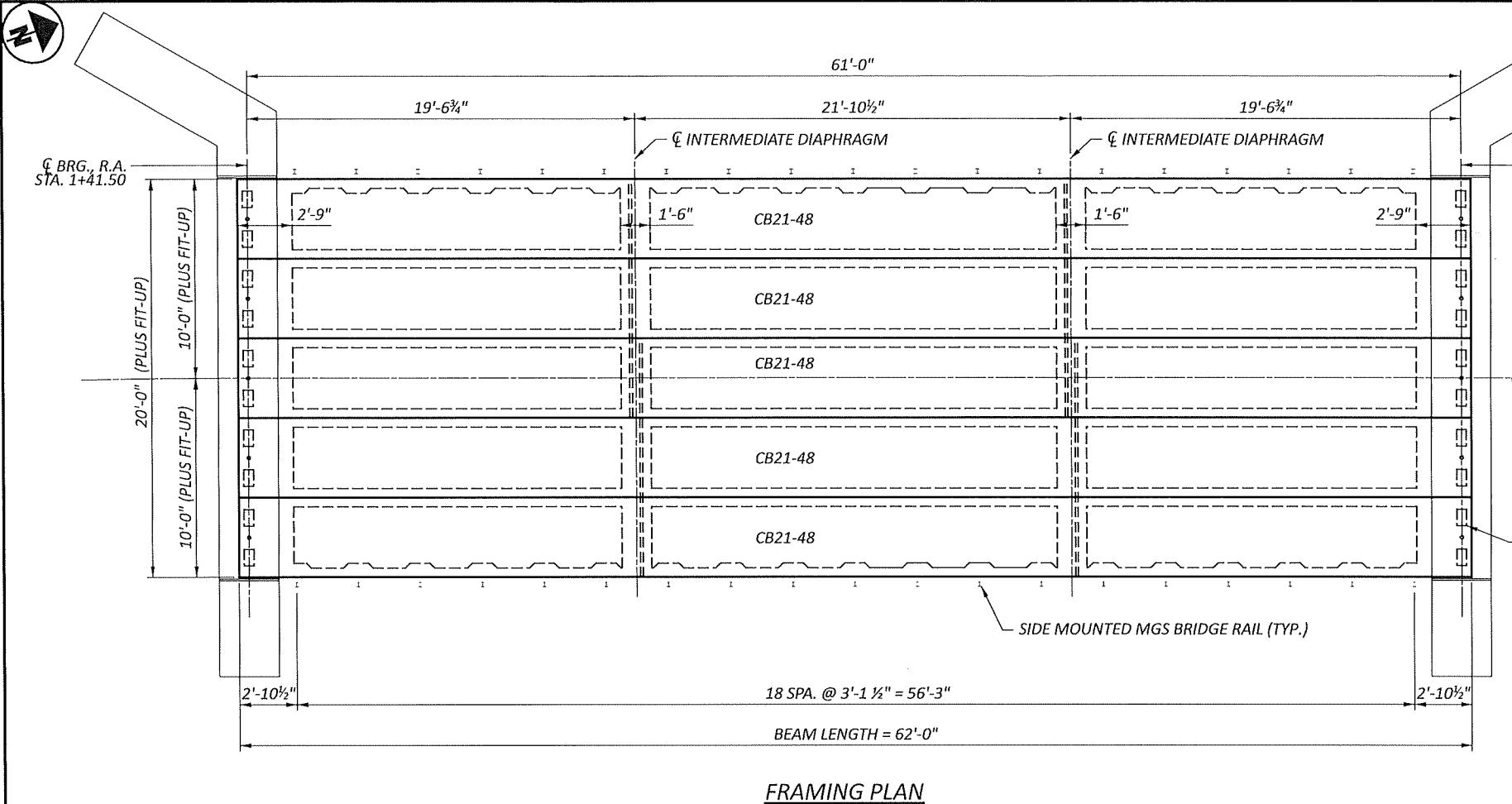
REVIEWER

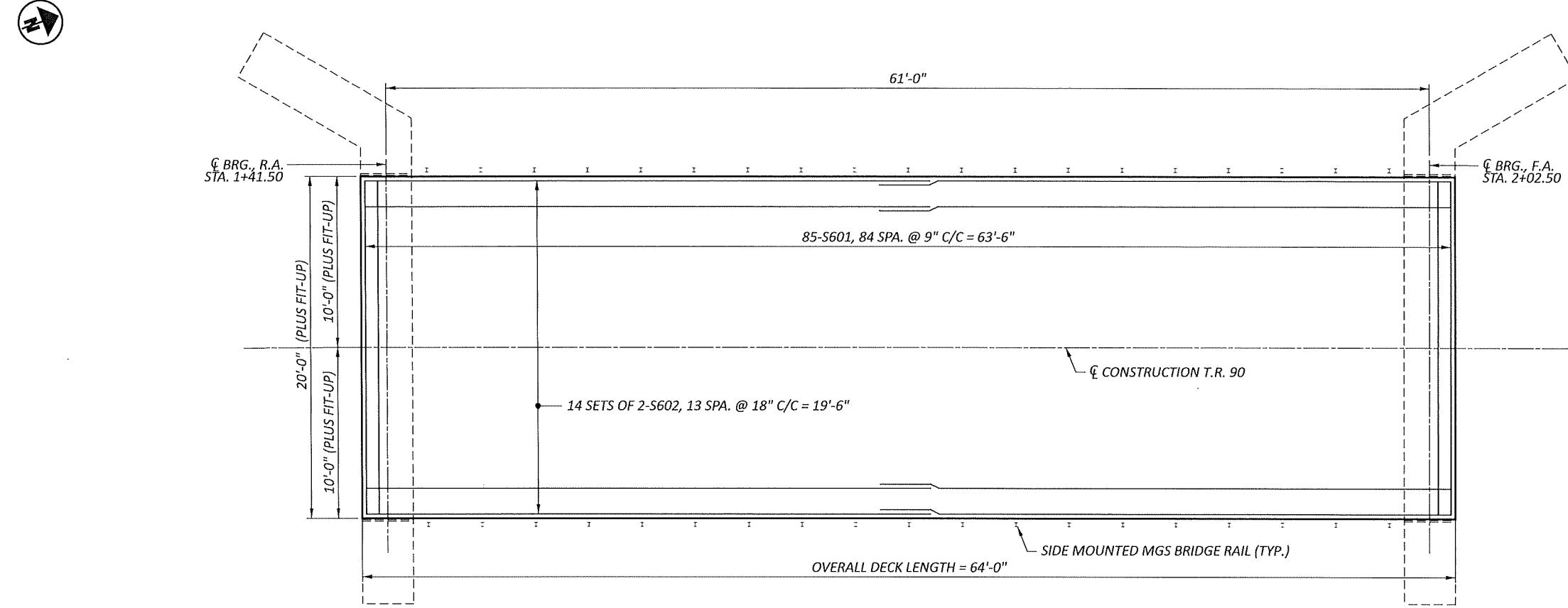
JECT ID

SET TOTAL

6 9
65 101M

15 | 18





DECK PLAN

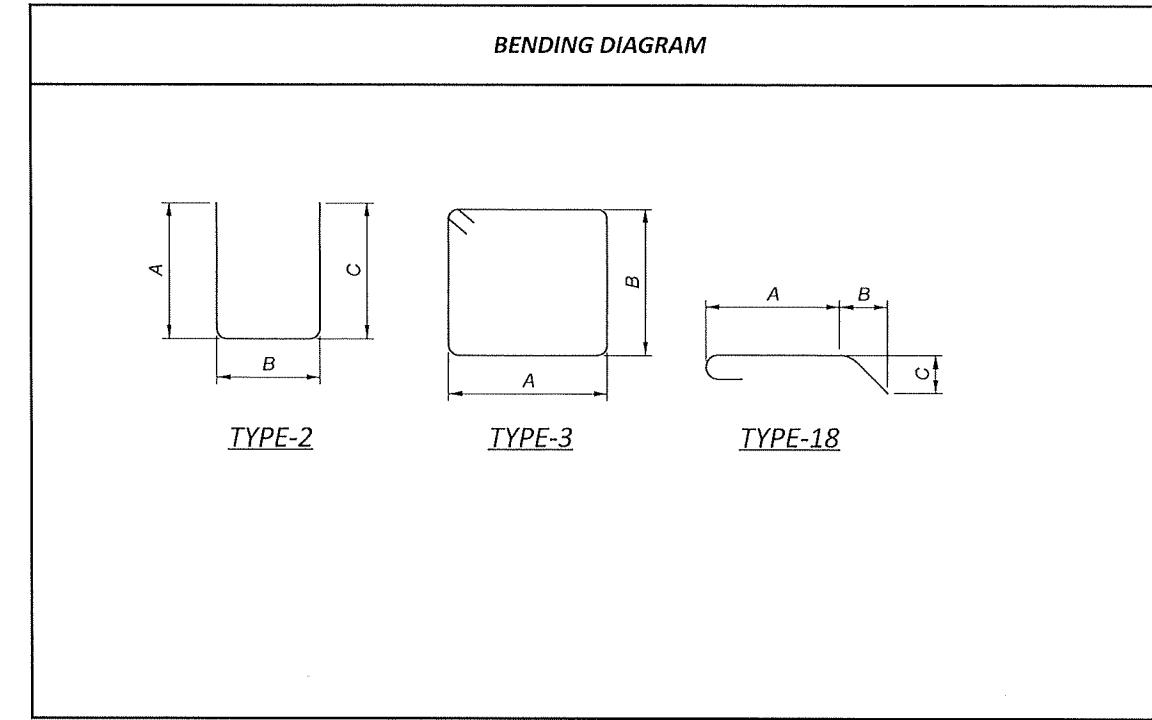
NOTES:

1. ALL REINFORCING STEEL SHALL BE EPOXY COATED.
2. MAINTAIN A 3" CLEARANCE TO THE EDGE OF DECK FOR ALL TRANSVERSE REINFORCING STEEL.
3. MINIMUM LAP SPLICES:
#4 BAR = 2'-3"
#5 BAR = 2'-11"
4. SEE SHEET **[7/9]** FOR TRANSVERSE SECTION.
5. SCREED ELEVATIONS SHALL BE CALCULATED AND PROVIDED BY THE LOAD RATING CONSULTANT. SCREED ELEVATIONS REPRESENT THE THEORETICAL DECK SURFACE ELEVATIONS PRIOR TO DEFLECTIONS CAUSED BY DECK PLACEMENT AND OTHER ANTICIPATED DEAD LOADS.

DECK PLAN
BRIDGE NO. MUS-TR90-0400
OVER THOMPSON RUN

SFN	6038396
DESIGN AGENCY	
SEJ	GJW
REVIEWER	GJW 3/20/23
PROJECT ID	117331
SUBSET	TOTAL
8	9
SHEET	TOTAL
17	18

MARK	NUMBER			LENGTH	WEIGHT	TYPE	DIMENSIONS						
	REAR	FWD	TOTAL				A	B	C	D	E	R	INC
ABUTMENTS													
A401	12	12	24	8'-11"	143	3	1'-9"	2'-6"					
A501	50	50	100	13'-1"	1365	2	5'-4"	2'-8"	5'-4"				
A502	22	22	44	10'-5"	478	2	4'-0"	2'-8"	4'-0"				
A503	8	8	16	28'-0"	467	STR							
A504	16	16	32	9'-8"	323	STR							
A505	6	6	12	4'-4"	54	STR							
A506	6	6	12	2'-9"	34	STR							
A801	8	8	16	28'-0"	1196	STR							
A802	4	4	8	9'-8"	206	STR							
SUB-TOTAL				4,266									



MARK	NUMBER			LENGTH	WEIGHT	TYPE	DIMENSIONS						
	REAR	FWD	TOTAL				A	B	C	D	E	R	INC
DECK													
S501	14	14	28	5'-4"	156	3	1'-2"	1'-2"					
S502	14	14	28	4'-1"	119	2	1'-10"	0'-8"	1'-10"				
S503	4	4	8	19'-6"	163	STR							
S601			28	33'-7"	1413	STR							
S602			86	19'-6"	2519	STR							
S801	14	14	28	4'-3"	318	18	2'-0"	1'-1"	1'-0"				
SUB-TOTAL				4,688									

NOTES

1. ALL REINFORCING STEEL SHALL BE EPOXY COATED.
2. THE BAR SIZE NUMBER IS SPECIFIED ON THE PLANS IN THE BAR MARK COLUMN, THE FIRST DIGIT WHERE THREE DIGITS ARE USED, AND THE FIRST TWO DIGITS WHERE FOUR DIGITS ARE USED, INDICATES THE BAR SIZE NUMBER. FOR EXAMPLE, A501 IS A NO. 5 BAR. BAR DIMENSIONS SHOWN ARE OUT TO OUT UNLESS OTHERWISE INDICATED.

PROJECT: WORTMAN RD. BRIDGE		DRILLING FIRM / OPERATOR: DHDC / DONALD		DRILL RIG: CME 55 TRUCK		STATION / OFFSET: _____		EXPLORATION ID B-1				
TYPE: ROADWAY		SAMPLING FIRM / LOGGER: DHDC / M.O.H.		HAMMER: CME AUTOMATIC		ALIGNMENT: _____						
PID: SFN: 6038-395		DRILLING METHOD: _____		CALIBRATION DATE: 7/21/14		ELEVATION: 0.0 (MSL)		EOB: 45.0 ft.				
START: 5/7/15 END: 5/7/15		SAMPLING METHOD: _____		COORD: Not Recorded		PAGE 1 OF 1						
MATERIAL DESCRIPTION AND NOTES		ELEV. 0.0	DEPTHs		SPT/ RQD	N ₆₀ (%)	REC SAMPLE ID	HP (tsf)	GRADATION (%)	ATTERBERG	ABAN-	DONED
FILL: A mixture of SAND and GRAVEL, SANDSTONE fragments, and tree roots, Moist		-1.2	-2.6		3 8	21 67	SS-1	-	GR CS FS SI CL	LL PL PI	WC	QDOT CLASS (G)
FILL: Dark brown, SILTY SAND and ROCK fragments, Moist		-3.0	-5.0		4 3	9 100	SS-2	-	-	-	-	-
Loose, Brown and gray, SANDY SILT (A-4a), with trace to little gravel, Moist		-5.0	-6.5		7 67	SS-3	-	-	-	-	-	-
Soft, Brown and gray, SILTY CLAY (A-6b), with trace to little gravel, Moist		-6.5	-7.5		1 7	83	SS-4	-	-	-	-	23
Soft, Dark brown, SILTY CLAY (A-6b), with trace sand, Moist		-7.5	-10.0		2 5	100	SS-5	-	-	-	-	22
Loose, Brown and gray, SANDY SILT (A-4b), Moist		-11.0	-12.5		3 12	100	SS-6	-	-	-	-	25
Medium dense, SAND with SANDSTONE fragments, with little silt and clay (A-1-b), Very moist		-12.5	-15.0		2 21	100	SS-7	-	-	-	-	19
Very stiff to hard, Brownish gray to gray, CLAY (A-7-b), [extremely weathered soft, SHALE], and interbedded SANDSTONE fragments, Very moist		-15.0	-17.5		3 6	28	67	SS-8	-	-	-	13
Interbedded layers of extremely weathered soft, SHALE and hard SANDSTONE		-17.5	-20.0		8 43	83	SS-9	-	-	-	-	14
		-20.0	-22.5		10 11	52	67	SS-10	-	-	-	18
		-22.5	-25.0		10 14	52	67	SS-11	-	-	-	13
		-25.0	-27.5		13 26	-	100	SS-12	-	-	-	18
		-27.5	-30.0		60/4"	-	100	SS-13	-	-	-	-
		-30.0	-32.5		60/3"	-	100	SS-14	-	-	-	-
		-32.5	-35.0		60/3"	-	100	SS-15	-	-	-	-
		-35.0	-37.5		60/2"	-	100	SS-16	-	-	-	-
		-37.5	-40.0		60/2"	-	100	SS-17	-	-	-	-
		-40.0	-42.5		60/2"	-	100	SS-18	-	-	-	-
		-42.5	-45.0		60/2"	-	100	SS-19	-	-	-	-
Boring discontinued at 45.0 feet due to auger refusal.		EOB 45.0										
NOTES: BOTTOM OF CREEK 8.0 FEET BELOW THE BRIDGE DECK. 12" OF WATER IN THE CREEK.												
ABANDONMENT METHODS, MATERIALS, QUANTITIES: NOT RECORDED												

PROJECT: WORTMAN RD. BRIDGE		DRILLING FIRM / OPERATOR: DHDC / DONALD		DRILL RIG: CME 56 TRUCK		STATION / OFFSET: _____		EXPLORATION ID B-2				
TYPE: ROADWAY		SAMPLING FIRM / LOGGER: DHDC / M.O.H.		HAMMER: CME AUTOMATIC		ALIGNMENT: _____						
PID: SFN: 6038-395		DRILLING METHOD: _____		CALIBRATION DATE: 7/21/14		ELEVATION: 0.0 (MSL)		EOB: 44.0 ft.				
START: 5/7/15 END: 5/7/15		SAMPLING METHOD: _____		COORD: Not Recorded		PAGE 1 OF 1		Not Recorded				
MATERIAL DESCRIPTION AND NOTES		ELEV. 0.0	DEPTHs		SPT/ RQD	N ₆₀ (%)	REC SAMPLE ID	HP (tsf)	GRADATION (%)	ATTERBERG	ABAN-	DONED
ASPHALT PAVEMENT (8")		-0.7	-1.3		6 4	9 67	SS-1	-	-	-	-	-
GRANULAR BASE (8")		-1.3	-3.0		2 2	5 100	SS-2	-	-	-	-	16
FILL: Dark brown, SILTY SAND and ROCK fragments, Moist		-3.0	-5.0		2 7	100	SS-3	-	-	-	-	18
Loose, Brown and gray, SANDY SILT (A-4a), with trace to little gravel, Moist		-5.0	-7.5		2 8	72	SS-4	-	-	-	-	21
Medium stiff, Dark brown with trace gray, SILTY CLAY (A-6b), with little sand, Moist		-7.5	-10.0		3 8	100	SS-5	-	-	-	-	29
Gray, SILTY CLAY, with trace sand, trace decomposed organics and odor, Very moist to wet		-10.0	-11.5		3 4	100	SS-6	-	-	-	-	34
Dense, SAND with SANDSTONE fragments, with little silt and clay (A-1-b), Very moist		-11.5	-15.0		3 63	72	SS-7	-	-	-	-	9
Hard, Brownish gray to gray, CLAY (A-7-b), [extremely weathered soft, SHALE], and interbedded SANDSTONE fragments, Moist		-15.0	-17.5		1 55	89	SS-8	-	-	-	-	-
Interbedded layers of extremely weathered soft, SHALE and hard SANDSTONE		-17.5	-20.0		12 35	-	100	SS-9	-	-	-	-
		-20.0	-22.5		20 50	-	100	SS-10	-	-	-	15
		-22.5	-25.0		60/3"	-	100	SS-11	-	-	-	-
		-25.0	-27.5		60/3"	-	100	SS-12	-	-	-	-
		-27.5	-30.0		60/4"	-	50	SS-13	-	-	-	-
		-30.0	-32.5		60/4"	-	50	SS-14	-	-	-	-
Boring discontinued at 44.0 feet due to auger refusal.		EOB 44.0										
NOTES: BOTTOM OF CREEK 8.0 FEET BELOW THE BRIDGE DECK. 12" OF WATER IN THE CREEK.												
ABANDONMENT METHODS, MATERIALS, QUANTITIES: NOT RECORDED												

STRUCTURE FOUNDATION EXPLORATION
TOWNSHIP ROAD 70 OVER THOMPSON RUN

DESIGN AGENCY
MUSKINGUM COUNTY ENGINEER'S OFFICE

DESIGNER SEJ
REVIEWER MJE 3/20/23
PROJECT ID 117331
SUBSET TOTAL 1 1
SHEET TOTAL

PLAN INSERTION SHEET
RAILING SYSTEM
SIDE MOUNTED MGS

DESIGNER

SEJ
REVIEWER

GJW 3/20/2

117331

1 1

SHEET TOTAL

1000

ROADWAY ELEVATION OF RAIL

W-BEAM ELEVATION

BACK VIEW

MOUNTING BRACKET

ELEVATION SIDE VIEW (Backer Plate)

POST ELEVATION

KENTUCKY DEPARTMENT OF HIGHWAYS RAILING SYSTEM SIDE MOUNTED MGS DETAILS

STANDARD DRAWING NO. BHS-011

TRANSITION AND END TREATMENT NOTES:
This traffic railing must be anchored by a minimum of 25 feet of guardrail. This 25 feet at each corner of the bridge is to be paid with the roadway plans. See roadway plans for layout.

CONSTRUCTION NOTES:
Face of rail post must be plumb unless otherwise approved by the Engineer. Post must be perpendicular to adjacent roadway grade. Fully anchored guardrail must be attached to each end of rail. Typical guardrail construction as indicated above and not bridge rail transition or bridge end connector. It is recommended that the bridge plans show rail post locations. Round or chamfer exposed edges of rail posts and backer Plate to approximately $\frac{1}{8}$ " by grinding. Shop drawings are not required. Threaded rod may be cast in the beam/slab or may be drilled and epoxy grouted. Epoxy grout must conform to Section 826 and must have a minimum bond strength of 1,305 psi. Follow all manufacturer's recommendations for installation.

MATERIAL NOTES:
All components must be supplied galvanized including fasteners, anchor rods, threaded rods, etc. Galvanize all steel components after fabrication in accordance with ASTM A123. W-beam must meet the requirements of Std. Dwg. RBR-001, c.e. except as modified in these plans. The contractor may furnish rail elements of 25'-0" or 12'-6" (Nominal) lengths. W-beam must have slotted holes at 3-1/2".

GENERAL NOTES:
This railing has been successfully evaluated by full scale crash test to meet MASH TL-3 criteria. This railing can be used for speeds 45 mph and greater. This rail is designed to deflect approximately 4'-0" - 4'-6" as it contains and redirects the errant vehicle. This rail may not be installed on top of or behind curbs that project above finished grade, on bridges with expansion joints providing more than 5" of movement, on retaining walls, or on grade separations and interchanges. Repairs to impact-damaged post and mounting bracket unit are not permitted. Replace all impact-damaged posts with a new post. If mounting bracket is visibly damaged, replace the bracket with a new one as well. Average weight of railing with no overlay: 19 plf total.

Note: Fabricator may elect to provide a $\frac{3}{4}$ " ASTM A563A heavy hex sleeve nut with a minimum length of $2\frac{1}{4}$ " and minimum $1\frac{1}{4}$ " across the flats along with threaded rod and the Anchor Rod. Maintain $12"$ minimum embedment with the $\frac{3}{4}$ " Anchor Rod behind the sleeve nut. The fabricator may elect to provide a $\frac{1}{2}$ " ferrule insert for the bottom anchors with a minimum length of $2\frac{3}{4}$ " and a safe working load of 2000 lbs in tension and shear along with the threaded rod. Alternatively, the bottom anchors may also be supplied with a $\frac{1}{2}$ " ASTM A563A heavy hex sleeve nut with a minimum length of $1\frac{1}{2}$ " and a minimum of $\frac{7}{8}$ " across the flats. Maintain $4\frac{1}{2}$ " embedment behind the sleeve nut. All costs for sleeve nuts, threaded rods, anchor rods, etc. are incidental to the price bid for the Railing System Side Mounted MGS.

**KENTUCKY
DEPARTMENT OF HIGHWAYS**

**RAILING SYSTEM
SIDE MOUNTED MGS
DETAILS**

STANDARD DRAWING NO. BHS-011

SUBMITTED By [Signature] 02-26-20
DIRECTOR DIVISION OF STRUCTURAL DESIGN DATE

APPROVED By [Signature] 02-26-20
STATE HIGHWAY ENGINEER DATE